

THURSTON COUNTY, WASHINGTON AND INCORPORATED AREAS

Preliminary: December 19, 2014

Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER 53067CV000B

NOTICE TO FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study (FIS) report may not contain all data available within the Community Map Repository. Please contact the Community Map Repository for any additional data.

The Federal Emergency Management Agency (FEMA) may revise and republish part or all of this FIS report at any time. In addition, FEMA may revise part of this FIS report by the Letter of Map Revision process, which does not involve republication or redistribution of the FIS report. Therefore, users should consult with community officials and check the Community Map Repository to obtain the most current FIS report components.

Selected Flood Insurance Rate Map (FIRM) panels for this community contain information that was previously shown separately on the corresponding Flood Boundary and Floodway Map (FBFM) panels (e.g., floodways, cross sections). In addition, former flood hazard zone designations have been changed as follows:

Initial Countywide FIS Effective Date: October 16, 2012

Revised Countywide Study: To Be Determined

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FLOOD INSURANCE STUDY THURSTON COUNTY, WASHINGTON AND INCORPORATED AREAS

1.0 INTRODUCTION

1.1 Purpose of Study

This Flood Insurance Study (FIS) investigates the existence and severity of flood hazards in, or revises and updates previous FIS reports/Flood Insurance Rate Maps (FIRMs) for the geographic area of Thurston County, including the Cities of Lacey, Olympia, Rainier, Tenino, Tumwater, Yelm; the Town of Bucoda, and the unincorporated areas of Thurston County (hereinafter referred to collectively as Thurston County). Within Thurston County, the Nisqually Indian Reservation is not participating in the NFIP.

This FIS aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This study has developed flood-risk data for various areas of the county that will be used to establish actuarial flood insurance rates. This information will also be used by the communities of Thurston County to update existing floodplain regulations as part of the Regular Phase of the National Flood Insurance Program (NFIP), and by local and regional planners to further promote sound land use and floodplain development. Minimum floodplain management requirements for participation in the NFIP are set forth in the Code of Federal Regulations at 44 CFR, 60.3.

In some States or communities, floodplain management criteria or regulations may exist that are more restrictive or comprehensive than the minimum Federal requirements. In such cases, the more restrictive criteria take precedence, and the State (or other jurisdictional agency) will be able to explain them.

The Digital Flood Insurance Rate Map (DFIRM) and FIS report for this countywide study have been produced in digital format. Flood hazard information was converted to meet the Federal Emergency Management Agency (FEMA) DFIRM database specifications and Geographic Information System (GIS) format requirements. The flood hazard information was created and is provided in a digital format so that it can be incorporated into a local GIS and be accessed more easily by the community.

1.2 Authority and Acknowledgments

The sources of authority for this FIS report are the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

Precountywide FIS Report

No previous FIS report was prepared for the City of Rainier, or the Nisqually Reservation; therefore the previous authority and acknowledgment information for these communities are not included in this FIS. These communities may not appear in the Community Map History table (Section 6).

October 16, 2012 Initial Countywide FIS Report

For the October 16, 2012, countywide study, all flooding sources studied by detailed methods were redelineated on new topographic data derived from the 2002 Puget Sound LiDAR Consortium (PSLC) Bare Earth LiDAR ASCII Points data, developed by TerraPoint, Inc. The LiDAR data has a Root Mean Square (RMS) vertical accuracy of approximately 30 centimeters.

In addition, the Nisqually River was converted to approximate zone due to the extreme stream channel migration occurring since the original models where developed.

The Deschutes River floodway and floodway data tables were removed, also due to the extreme channel migration within the floodplain.

Approximate areas were spatially adjusted to the new base maps, as necessary.

The orthophotography base mapping was provided in digital format by Thurston County Geodata Center, Washington Department of natural Resources (DNR) and USGS. This information was compiled at scales of 1:2,400 to 1:24,000 during the time period of 1996 to 2007. The digital countywide Flood Insurance Rate Map (FIRM) was produced in Washington State Plane South Zone (FIPS Zone 4602) coordinate system with a Lambert Conformal Conic projection, units in feet, and referenced to the North American Datum of 1983, GRS80 spheroid. Differences in datum and spheroid used in the production of the FIRMs for adjacent counties may result in slight positional differences in map features at the county boundaries. These differences do not affect the accuracy of information shown on this FIRM.

This Physical Map Revision

For this physical map revision, the Deschutes River and select Zone A areas were restudied to include the use of newly acquired topography in the hydraulic analyses. The study was completed using approximate hydraulic analysis involving the use of LiDAR data and HEC-RAS hydraulic modeling software. The hydraulic analyses for this study were performed by the Strategic Alliance for Risk Reduction (STARR) for FEMA, under contract number HSFEHQ-09-D-0370, Task Order No. HSFE10-10-J-00106. The work was completed in March 2013.

Base map information shown on the FIRM was derived from the National Agriculture Imagery Program dated 2011. This study is a vector based study and will not have an aerial photo associated with the base map submission.

1.3 Coordination

An initial Consultation Coordination Officer's (CCO) meeting is held typically with representatives of FEMA, the community, and the study contractor to explain the nature and purpose of a FIS and to identify streams to be studied by detailed methods. A final CCO meeting is held typically with the same representatives to review the results of the study. The initial and final meeting dates for the previous FIS reports for Thurston County and its communities are listed in Table 1, "Initial and Final CCO Meetings".

Precountywide FIS Report

Table 1 – Initial and Final CCO Meetings

October 16, 2012 Initial Countywide FIS Report

For the October 16, 2012, revision, the final CCO meeting was held on September 30, 2010 with representatives of FEMA, Michael Baker Jr. Inc., and representatives from the local communities of the Cities of Lacey, Olympia, Rainier, Tumwater, Yelm and Thurston County. All problems raised at that meeting have been addressed.

This Physical Map Revision

The FEMA Region X Watershed Discovery Meeting was held on October 2010, and attended by representatives of FEMA, Washington DNR, STARR and the communities.

A final CCO meeting was held on ____________, and was attended by representatives of the

2.0 AREA STUDIED

2.1 Scope of Study

This FIS covers the geographic area of Thurston County, Washington, including the incorporated communities listed in Section 1.1.

October 16, 2012 Initial Countywide FIS Report

The areas studied by detailed methods were selected with priority given to all known flood hazards and areas of projected development or proposed construction. Table 2, "Streams Studied by Detailed Methods", lists the flooding sources which were studied by detailed methods and redelineated based on updated topography. Limits of detailed study are indicated on the Flood Profiles (Exhibit 1) and on the FIRM (Exhibit 2).

Table 2 – Streams Studied by Detailed Methods

Lakes and bays studied in detail include: Black Lake, Bigelow Lake, Budd Inlet, Capitol Lake, Chambers Lake, Clear Lake, Hicks Lake, Ken Lake, Lake Lawrence, Long Lake, Nisqually Reach, Pattison Lake, Setchfield Lake, Summit Lake, Tempo Lake, and Trosper Lake.

Table 3, "Areas Studied by Approximate Methods", lists the flooding sources which were studied by approximate methods.

Table 3 – Streams Studied by Approximate Methods

Community Limits of Study

Thurston County, Unincorporated Areas Alder Lake, Bald Hill Lake, Barnes Lake, Beatty Creek, Beaver Creek, Beaver Creek tributaries, Black Lake tributaries, Black River downstream of Black Lake, Black River Tributaries, Blooms Ditch, Blooms Ditch Overflow to Salmon Creek, Chapman Run, Chehalis River areas along detailed study, Chehalis River Overflows, Chehalis River tributary, Coffee Creek, Coffee Creek West Branch, Deep Lake, Dempsey Creek, Deschutes River areas along detailed study, Deschutes River Overflows, Deschutes River tributaries, D'Miller Lake, Dry Creek, Eaton Creek, Eaton Creek Tributary, Edna Creek, Elbow Lake, Eld Inlet, Fry Cove, Gehrke Lake, Goose Pond, Grass Lake Outlet, Green Cove, Henderson Inlet, Henderson Inlet – Chapman Bay, Henderson Inlet – Woodward Bay, Indian Creek, Inmen Lake, Johnson Creek, Kennedy Creek, Lackamas Creek, Lagrande Reservoir, Lake Lawrence Outlet, Lake Lois, Lake Saint Clair, Laramie Creek Tributary, Little Deschutes River, Little Nisqually River, Long Lake Tributary, McAllister Creek, McAllister Creek Tributary, McLane Creek, McIntosh Lake, Medicine Creek, Mima Creek, Munn Lake, Nisqually River, North Hanaford Creek, Offutt Lake, Outlet of Black Lake Drainage Ditch, Outlet of Black Lake Tributary, Outlet of Grass Lake, Oyster Bay, Pattison Lake, Pattison Lake North, Pipeline Creek, Pitman Lake, Powell Creek, Puget Sound, Puget Sound – Big Fishtrap Cove, Reichel Lake, Reichel Lake Outlet, Salmon Creek, Scatter Creek (downstream of Tenino), Scatter Creek Tributaries, Scott Lake, Sheehan Lake, Skookumchuck Reservoir, Skookumchuck River (portions), Southwick Lake, Spurgeon Creek, Spurgeon Creek tributaries, Summit Lake Outlet, Susan Lake, Thompson Creek, Thompson Creek Overflow to Skookumchuck River, Thompson Creek Tributary, Toboton Creek, Totten Inlet, Trails End Lake, Trosper Lake, Waddell Creek, Ward Lake, Woodland Creek downstream of Pleasants Road SE, Woodward Creek, Yelm Creek outside of the Yelm City limits, Yelm Ditch, Young Cove, and numerous isolated ponding areas throughout the county. Town of Bucoda Skookumchuck River – Front Street Overflow, along the Burlington Northern Railroad in the vicinity of Main and Martina Streets

- City of Lacey Woodland Creek, upstream of Interstate Highway 5, and several unnamed ponding areas.
- City of Olympia Ellis Creek, Grass Lake, Grass Lake Outlet, Indian Creek, Ken Lake Tributary East, Ken Lake Tributary West, Mission Creek, Outlet of Black Lake, Percival Creek, Percival Cove, Setchfield Lake, Ward Lake, Woodward Creek, and various unnamed ponding areas.

City of Tenino Scatter Creek Tenino Tributary 1 and Scatter Creek Tenino Tributary 2

Table 3 – Streams Studied by Approximate Methods *(Continued)*

Approximate methods of analyses were used to study those areas having a low development potential or minimal flood hazards.

Approximate methods of analyses were used to study those areas having a low development potential or minimal flood hazards. The scope and methods of study were proposed to and agreed upon by the FEMA and the study contractor.

Town of Bucoda

Approximate analyses were performed by field survey and engineering judgment. The Skookumchuck River – Front Street Overflow, along the Burlington Northern Railroad in the vicinity of Main and Martina Streets was studied by approximate methods.

City of Lacey

Approximate methods of analyses were used to study those areas having a low development potential or minimal flood hazards. Woodland Creek, upstream of Interstate Highway 5, and several swampy areas were studied by approximate methods.

City of Olympia

Shallow flooding or ponded areas studied by approximate methods were Percival Cove and an area north of Setchfield Lake. Riverine flooding was studied by approximated methods along Percival Creek, from Percival Cove upstream to the corporate limits and from Mottman Road Southwest upstream to the corporate limits; Ellis Creek northeast of East Bay Drive; and Indian Creek, from Interstate Highway 5 upstream to the corporate limits. Additional streams are listed in Table 3.

Approximate methods of analyses were used to study those areas having a low development potential or minimal flood hazards. The scope and methods of study were proposed to and agreed upon by the FEMA and the City of Olympia.

City of Tumwater

Shallow flooding or ponded areas of Barnes Lake were studied by approximate methods. These included areas west of Tumwater Junior High School; east of Miner Drive Southwest; south of Trosper Road in the vicinity of Schoth Street; south of Hartman Street and north of the Union Pacific Railroad tracks; north of Trosper Road and west of Lake Park Road; at the east end of E Street and east of the union Pacific Railroad; east of M, N, and O Streets; and, north of East T Street.

Two areas of riverine flooding were studied by approximate methods. These include Percival Creek, upstream from U.S. Highway 101 to Mottman Road and from Dacatur Street Southwest to Sapp Road (Thurston County-Tumwater corporate limits); and Deschutes River, from Capitol Lake upstream to the dam at Olympia Brewery.

City of Tenino

Shallow flooding areas studied by approximate methods were; ditch from culvert outfall at schoolyard to Scatter Creek, north of Garfield Avenue, fields north of Sussex Avenue, from Reynolds Street to Olympia-Tenino highway, and a residential area from Olympia-Tenino Highway to Custer Street.

Nisqually Reservation

Within the Thurston County portion of the Nisqually Reservation, the Nisqually River is unstudied. Effective work maps for the Nisqually River exclude the portion of the floodplain within the reservation. These areas are mapped as Zone D. Nisqually Reservation is a non-participating community.

Thurston County, Unincorporated Areas

Some overflow areas of the Black, Chehalis, Deschutes, and Skookumchuck Rivers; Indian, a portion of Woodland, a portion of Percival, Mima, Scatter, Waddell, Dempsey, Johnson, Thompson, Spurgeon, North Hanaford, and Toboton Creeks; the outlets of Grass Lake and Reichel Lake were studied by approximate methods.

Offut, Barnes, Sheehan, Munn, Susan, and Trails End Lakes; Totten, Eld, and Henderson Inlets; and Puget Sound along the coast of Thurston County were also studied by approximate methods.

The October 16, 2012, countywide FIS incorporates the determinations of Letter of Map Revisions (LOMRs) issued by FEMA, for the projects listed by community in Table 4, "Letters of Map Change (LOMCs)".

One LOMR (89-10-06P) was superseded based on engineering judgment during the floodplain redelineation using updated LiDAR topographic data. Another LOMR (94- 10-058P) was superseded due to insufficient information. The Capitol Lake LOMR (03- 10-0337P) was incorporated with the associated base flood elevation (BFE) change from 14 feet to 15 feet North American Vertical Datum of 1988 (NAVD88), but was redelineated on new LiDAR-derived elevation data.

This Physical Map Revision

The Deschutes River was restudied by detailed methods and the Deschutes River Tributaries 3, 3.1, 9, 11, 12, 14, 16, and 17, Little Deschutes River, Spurgeon Creek, and Offut Lake were restudied by approximate methods for this Physical Map Revision. No new LOMRs were incorporated.

FIRM Notes to Users

Each FIS report provides floodplain data, which may include a combination of the following: 10-, 4-, 2-, 1-, and 0.2-percent-annual-chance flood elevations (the 1 percent-annual-chance flood elevation is also referred to as the Base Flood Elevation (BFE)); delineations of the 1-percent-annual-chance and 0.2-percent- annual-chance floodplains; and 1-percent-annual-chance floodway. This information is presented on the FIRM and/or in many components of the FIS report, including Flood Profiles and Floodway Data tables.

Figure 1 presents important considerations for using the information contained in this FIS report and the FIRM and is provided in response to changes in format and content.

Figure 1 - FIRM Notes to Users

NOTES TO USERS

For information and questions about this map, available products associated with this FIRM including historic versions of this FIRM, how to order products, or the National Flood Insurance Program in general, please call the FEMA Map Information eXchange at 1-877-FEMA-MAP (1- 877-336-2627) or visit the FEMA Map Service Center website at http://msc.fema.gov. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the website. Users may determine the current map date for each FIRM panel by visiting the FEMA Map Service Center website or by calling the FEMA Map Information eXchange.

Communities annexing land on adjacent FIRM panels must obtain a current copy of the adjacent panel as well as the current FIRM Index. These may be ordered directly from the Map Service Center at the number listed above.

For community dates, refer to Table 14 in this FIS Report.

To determine if flood insurance is available in the community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

PRELIMINARY FIS REPORT: FEMA maintains information about map features, such as street locations and names, in or near designated flood hazard areas. Requests to revise information in or near designated flood hazard areas may be provided to FEMA during the community review period, at the final Consultation Coordination Officer's meeting, or during the statutory 90-day appeal period. Approved requests for changes will be shown on the final printed FIRM.

The map is for use in administering the NFIP. It may not identify all areas subject to flooding, particularly from local drainage sources of small size. Consult the community map repository to find updated or additional flood hazard information.

BASE FLOOD ELEVATIONS: For more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables within this FIS Report. Use the flood elevation data within the FIS Report in conjunction with the FIRM for construction and/or floodplain management.

FLOODWAY INFORMATION: Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the FIS Report for this jurisdiction.

Floodways restricted by anthropogenic features such as bridges and culverts are drawn to reflect natural conditions and may not agree with the model computed widths listed in the Floodway Data table in the Flood Insurance Study.

In the State of Illinois, any portion of a stream or watercourse that lies within the floodway fringe of a studied (AE) stream may have a state regulated floodway. The FIRM may not depict these state regulated floodways.

FLOOD CONTROL STRUCTURE INFORMATION: Certain areas not in Special Flood Hazard Areas may be protected by flood control structures.

PROJECTION INFORMATION: The projection used in the preparation of the map was State Plane Illinois West 1201. The horizontal datum was NAD83, GRS1980 spheroid. Differences in datum, spheroid, projection or State Plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of the FIRM.

ELEVATION DATUM: Flood elevations on the FIRM are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282 (301) 713-3242

Local vertical monuments may have been used to create the map. To obtain current monument information, please contact the appropriate local community.

BASE MAP INFORMATION: Base map information is panel-specific. The map panels should be referenced for this information.

Corporate limits shown on the map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after the map was published, map users should contact appropriate community officials to verify current corporate limit locations.

NOTES FOR FIRM INDEX

REVISIONS TO INDEX: As new studies are performed and FIRM panels are updated within Lake County, IL, corresponding revisions to the FIRM Index will be incorporated to reflect the effective dates of those panels.

FLOOD RISK REPORT: A Flood Risk Report (FRR) may be available for many of the flooding sources and communities referenced in this FIS Report. The FRR is provided to increase public awareness of flood risk by helping communities identify the areas within their jurisdictions that have the greatest risks. Although non-regulatory, the information provided within the FRR can assist communities in assessing and evaluating mitigation opportunities to reduce these risks. It can also be used by communities developing or updating flood risk mitigation plans. These plans allow communities to identify and evaluate opportunities to reduce potential loss of life and property. However, the FRR is not intended to be the final authoritative source of all flood risk data for a project area; rather, it should be used with other data sources to paint a comprehensive picture of flood risk.

2.2 Community Description

Thurston County is located in the west-central area of Washington, just south of Budd Inlet –a southern arm of Puget Sound. Thurston County is bordered by Mason County to the northwest, Gray's Harbor County to the immediate west, Lewis County to the south and Pierce County to the east.

Thurston County is comprised of seven incorporated communities (six cities, one town) and the unincorporated areas. According to the 2010 U.S. Census Bureau, the population for Thurston County was 252,264 with land and water area totaling 773.6 square miles (U.S. Census Bureau, 2014).

The climate of Thurston County is marine with an annual precipitation ranging from approximately 40 inches on the eastern lowland prairies to approximately 60 inches in the southeastern and northwestern hills. In the City of Olympia, the average annual precipitation is approximately 50 inches, 39 inches of which fall from October to March (The Weather Channel, 2014).

During summer months, the average monthly high is 75 degrees Fahrenheit (ºF) and the average monthly low is 50ºF. The highest temperatures are usually recorded during the month of August. During winter months, the average monthly high is 46ºF and the average monthly low is 33ºF. The lowest temperatures are usually recorded during the month of December (The Weather Channel, 2014).

The five main river systems in Thurston County are Nisqually, Deschutes, Black, Skookumchuck, and Chehalis Rivers. Black and Skookumchuck Rivers are major tributaries to Chehalis River (FEMA, Thurston County Unincorporated Areas, 1999).

Nisqually River meanders along Thurston County's eastern boundary with Pierce County. Deschutes River flows northwesterly for approximately 41 miles within Thurston County towards its mouth at Capitol Lake in Olympia. Black River is a slow, meandering stream that extends from Black Lake south for approximately 19 miles in Thurston County. Skookumchuck River extends for approximately 24.7 miles in south central Thurston County and has a wide flood plain from the county line upstream for 15 miles. Chehalis River extends for only 8.6 miles in Thurston County, but has an extensive flood plain, covering at least 12 square miles (Thurston Regional Planning Council, 1975).

2.3 Principal Flood Problems

Flooding in Thurston County has been a result of heavy rainfall, sometimes augmented with runoff contributions from snowmelt. Flooding generally occurs during the winter months, November through February, when storms bring intense precipitation. The major flood problems are those of inundation and damage to private property from out-of-bank floodwaters.

The history of flooding in Lacey indicates that flooding occurs along Woodland Creek, in local depressions and marshes, and along the lakes.

For the City of Olympia, rain coupled with storm-driven high tide has caused inundation and property damage. The business and industrial areas around Budd Inlet and Capitol Lake suffer the most damage, with additional impacts from the overflow of Outlet of Black Lake and Ken, Setchfield, and Chambers Lakes. A historical high tide (approximately 1-percent-annual-chance) occurred on December 15, 1977, when many

businesses along Budd Inlet and Capitol Lake were inundated.

Flood damage on Nisqually River in the unincorporated portions of Thurston County is generally limited to an area near McKenna in Pierce County. A discharge flow of approximately 18,000 cubic feet per second (cfs) at McKenna is associated with zeroflood damage on Nisqually River (Pacific Northwest River Basins Commission, 1970). This flow has been exceeded six times during the period of record (1947-78) at the USGS gaging station on Nisqually River below Powell Creek near McKenna (No. 12088400), at RM 31.6. The three most severe floods occurred in December 1975 (30,700 cfs), January 1965 (25,700 cfs), and January 1974 (23,200 cfs) (U.S. Department of the Interior, -1971, 1971-74, 1975-1978). The December 1933 flood, estimated at 42,000 cfs, inundated most of the delta (Pacific Northwest River Basins Commission, 1971).

Near the mouth of Deschutes River, a discharge of 3,600 cfs is considered to represent zero-damage flow (Pacific Northwest River Basins Commission, 1970). This flow has been exceeded at least 31 times between 1945 and 2007. On January 15, 1974, a flood with a recurrence interval of approximately 100 years occurred on the Deschutes River. The Tumwater Valley Golf Course was inundated, and the Olympia Brewing Company incurred some property damage during this flood. The most severe floods, as recorded at the gaging station on Deschutes River near Rainier (No. 12079000), at RM 25.9, are 9,600 cfs in January 1990, 7,850 cfs in February 1996, and 7,780 cfs in January 1974 (U.S. Department of the Interior, prior to 1971, 1971-74, 1975-1978).

No extensive records are available describing historic flooding on Black River. However, it is known that, during periods of flooding, Black River is inundated by floodwaters of Chehalis River as far as 5 miles upstream of the Thurston County limits (Thurston Regional Planning Council, 1975).

The three most severe floods on Skookumchuck River occurred in February 1996 (9,020 cfs), January 1990 (7,800 cfs), and December 1953 (6,710 cfs), as recorded by the gaging station below Bloody Run Creek (No. 12026150) (U.S. Department of the Interior, -1971, 1971-74, 1975-1978).

In December 2007, almost the entire Chehalis River flood plain was inundated by the largest flow (79,100 cfs) in 80 years (1928-2007) of record at the gaging station near Grand Mound (No. 12027500). The second and third most severe floods on the Chehalis River occurred in February 1996 (74,800 cfs), and January 1990 (68,700 cfs).

On February 8, 1996, an intense rainstorm occurred in Thurston County following several months of above-average precipitation. Eight inches of rain were recorded at the nearby Olympia Airport gage for the period from February 5-8, 1996. Observed rainfall at the Olympia gage for the period from November 1995 through January 1996 was approximately 40 percent higher than normal. Freezing temperatures and some snow accumulation were observed in the basin from late January through approximately February 4. This combination of meteorological inputs resulted in high flows and significant flooding along portions of Yelm Creek within the City of Yelm City limits.

Much of the floodplain along Yelm Creek was inundated, with large ponding areas upstream of several road crossings. Of the five roads crossed by Yelm Creek in the study reach, four were overtopped during the February 1996 event, including Crystal Springs Road, First Street, 103rd Avenue, and Bald Hills Road.

A slightly smaller flood event occurred from December 31, 1996, through January 2,

1997. Again, a moderately intense rainfall event occurred following an extended period of above-average precipitation. Just prior to this flood, significant snowfall accumulations were present over the entire Yelm Creek basin. The combination of high groundwater, rainfall runoff, and snowmelt caused high flows and significant flooding on Yelm Creek. It took several months for the water to recede, which indicates that the flooding was closely linked to high groundwater levels in the basin. Flooding throughout much of Thurston County was more severe for the December 1996 through January 1997 flood than for any event in recent history, although the February 1996 event was larger on Yelm Creek.

Prior to these two events, significant flooding occurred on Yelm Creek most recently in January 1990. Reports provided by the City of Yelm (Puget Land Consultants, 1994) indicate that the January 1990 flood overtopped at least one road in the study reach $(103rd)$ Avenue).

2.4 Flood Protection Measures

There are no physical flood protection measures in the Cities of Lacey, Olympia, and Yelm; and City of Tenino.

The Skookumchuck Dam, completed in 1971, is located on Skookumchuck River approximately 8 miles upstream of Bucoda and has a capacity of 42,000 acre-feet. Its major function is water supply for the Centralia Steam-Electric Project and provides little protection from large floods.

Two reservoirs with a combined storage capacity of 234,700 acre-feet (Alder Reservoir, 232,000 acre-feet, and LaGrande Reservoir, 2,700 acre-feet) are located in the Nisqually River basin. Firm flood-control storage is not provided by either reservoir, although the operation at Alder can be adjusted when a flood is expected to provide for 10,000 to 15,000 acre-feet of storage. This can reduce flood peaks on Nisqually River by an estimated 3,000 to 5,000 cfs (Pacific Northwest River Basins Commission, 1970). Several levees have been constructed on Nisqually, Chehalis, Deschutes, and Skookumchuck Rivers, but none are adequate to protect against the 1-percent-annualchance flood and are not shown on the maps.

Flood protective works consist of a levee and fill on the right bank of the Deschutes River at the Olympia Brewing Company, and stream revetments at several bridges. These structures were topped by the 1974 flood and offer little protection from floods greater than or equal to the 1-percent-annual-chance event. The Olympia Brewing Company Dam, located in the City of Tumwater, has no effect on flooding.

Limited regulation of flood plain development is provided by the Shoreline Master program of Thurston County and the Washington State Department of Ecology.

3.0 ENGINEERING METHODS

For the flooding sources studied by detailed methods in the community, standard hydrologic and hydraulic study methods were used to determine the flood hazard data required for this study. Flood events of a magnitude that are expected to be equaled or exceeded once on the average during any 10-, 2-, 1-, or 500-year period (recurrence interval) have been selected as having special significance for floodplain management and for flood insurance rates. These events, commonly termed the 10-, 2-, 1-, and 500-year floods, have a 10-, 2-, 1-, and 0.2-percent-annual- chance, respectively, of being equaled or exceeded during any year. Although the recurrence interval represents the long-term, average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood that equals or exceeds the 1-percent-annual-chance (100-year) flood in any 50-year period is approximately 40 percent (4 in 10); for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported herein reflect flooding potentials based on conditions existing in the community at the time of completion of this study. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Hydrologic Analyses

Hydrologic analyses were carried out to establish the peak discharge-frequency relationships for the flooding source studied by detail methods affecting the communities within Crawford County. Information on the methods used to determine the peak discharge-frequency relationships for each flooding source studied by detailed methods is shown below.

Precountywide Analysis

Hydrologic analyses were carried out to establish peak discharge-frequency relationships for each flooding source studied by detailed methods affecting the community.

For each community within Thurston County that had a previous printed FIS report, the unrevised hydrologic analysis described in those reports have been compiled and are summarized below by city or town.

In the Town of Bucoda, the peak discharge-frequency relationship for Skookumchuck River was computed from regression equations that relate peak discharge-frequency data to drainage area and mean annual precipitation. Fifty-one continuous-record streamgaging stations, with 6 to 47 years of peak-discharge records, and 14 peak-stage partial record stations, with 7 to 26 years of peak-discharge records, located mostly in Thurston and Pierce Counties, were used as the source of peak-discharge and drainage area data (U.S. Department of Interior, 1971, 1971-1974, 1975-1977). Precipitation data for each drainage basin were based on information from the U.S. Weather Bureau (U.S. Department of Commerce, 1965). Values of the 10-, 2-, 1-, 0.2-percent-annualchance peak discharges were obtained for the regression equations from a log-Pearson Type III distribution of annual peak discharges at each station in accordance with guidelines set forth in U.S. Water Resources Council Bulletin 17 (U.S. Water Resources Council,1976).

The possibility of using previously developed regional peak discharge-frequency relationships (U.S. Department of the Interior, 1964, 1975) was investigated before developing the regression equations used in this study. However, these relationships were not used because additional peak-discharge data have since become available, the log-Pearson Type III method of analysis has since been improved and standardized, and relationships for a smaller region were needed to more accurately reflect localized flood flow conditions.

In the City of Lacey, the regional relationships in existing publications (U.S. Department of the Interior, 1964; Collings, Cummans, Nassar, 1975) were compared to Woodland Creek relationships developed from gage data for the 10-, 2-, 1-, 0.2-percent-annualchance peak discharges. The regional relationships were not used because they do not define the local conditions. A series of lakes in the headwater temporarily stores water which decreases the peaks. For defining the peak discharge-frequency relationship, a USGS stream-gaging station on Woodland Creek, with a 19-year record (Collings et al., 1975), was used as the source of data. This station is located 1.25 miles downstream of the corporate limits. Values of the 10-, 2-, 1-, 0.2-percent-annual-chance peak discharges were obtained from a log-Pearson Type III distribution of annual peak flow data at this station in accordance with guidelines set forth by the U.S. Water Resources Council (1976). To represent the discharges of Woodland Creek at Draham Street NE, the station discharges were adjusted for the difference in drainage area at the station and at Draham Street NE by a power factor (0.8) found typical for western Washington streams.

Regional relationships used for several lakes in Lacey, Olympia, and Thurston County were developed for estimating the differences between mean lake elevation and the 10-, 2-, 1-, 0.2-percent-annual-chance peak elevations, based on log-Pearson Type III analysis of records (7 to 35 years in length) for nine lakes in western Washington with similar hydrologic settings ("Surface Water Supply", 1955, 1964, 1971; U.S. Department of the Interior, 1971-1974; "Water Resources Data", 1971-1974). These relationships were applied to determine the flood-peak elevations of Bigelow, Clear, Chambers, Hicks, Ken, Lawrence, Long, Pattison, Setchfield, Summit, and Tempo Lakes by adding difference values to lake elevations at time of photography in March and April 1977 (Walker and Associates), which were considered to be at the mean levels.

In the City of Olympia, Tumwater, and Tenino; fifty-one continuous-record streamgaging stations, with record lengths of 6 to 62 years, and 14 peak-stage, partial record stations, with from 7 to 26 years of peak data, from hydrologically similar sites (U.S. Department of the Interior, -1971; 1971-1974; 1975; 1976; 1977) were used as the source of data for defining the peak discharge-frequency relationship for Outlet of Black Lake and for each stream studied in the City of Tenino. Values of the 10-, 2-, 1-, 0.2-percentannual-chance peak discharges were obtained from a log-Pearson Type III distribution of annual peak flow data at these sites in accordance with the guidelines set forth in U.S. Water Resources Council Bulletin 17 (1976).

In the City of Olympia, tidal peak elevation-frequency relationship was developed by analyzing 71 years of annual peak tides, as recorded at the Seattle Tidal Station (U.S. Department of Commerce, 1975), with the log-Pearson Type III method, using +0.2 skew. Values of the 10-, 2-, 1-, 0.2-percent-annual-chance tidal peak elevations were then transferred to Olympia using the tide prediction tables (U.S. Department of Commerce, 1974). These relationships were applied to the Budd Inlet area.

Capitol Lake was created in 1951 by construction of an earth-fill dam on the intertidal estuary where Deschutes River and Percival Creek formerly joined Budd Inlet. Tide gates are used to fill the lake to approximately the elevation of the mean-higher-high tide, but an extreme high tide or riverflow can cause much higher elevations in the lake, just as they did in the former estuary. There is some difference between flood elevations for Capitol Lake and Budd Inlet, but elevations obtained during the extreme high tide of December 15, 1977, demonstrate that the difference is small. That difference was added to the 10-, 2-, 1-, 0.2-percent-annual-chance tidal elevations for Budd Inlet and used for Capitol Lake.

The analyses reported herein reflect the stillwater elevations due to tidal and wind setup effects, but do not include the contributions from wave action effects, such as the wavecrest height and wave runup. Nevertheless, this additional hazard due to wave action

effects should be considered in planning of future development.

Tidal and wind setup effects for Budd Inlet were determined by comparing the highwater mark elevations of the December 1977 storm against the recorded high tide levels as transferred from Seattle. These effects were added to the values of the 10-, 2-, 1-, 0.2 percent-annual-chance tidal peak elevations.

In the City of Tenino, regional relationships in existing publications (U.S. Department of the Interior, 1964; Magnitude and Frequency, 1975) did not produce satisfactory results for the 10-, 2-, 1-, 0.2-percent-annual-chance peak discharges in comparison with those obtained for the gaged sites by the log-Pearson Type III distribution. Therefore, new regional relationships of basin characteristics (drainage area and precipitation) to streamflow characteristics (10-, 2-, 1-, 0.2-percent-annual-chance peak discharges) were developed for determining peak discharges at all sites in the study areas. A list of published gage records used as the source of data for defining peak discharge-frequency relationship are listed below in Table 5, "USGS Gages Used in the Hydrologic Analysis."

Table 5 – USGS Gages used in Hydrologic Analysis

A total of 43 other continuous-record stream-gaging stations and 14 peak-stage partialrecord stations from hydrologically similar sites, most of which were in Pierce and Thurston Counties (U.S. Department of the Interior, -1971; 1971-74; 1975-78), were also used in the hydrologic analyses.

Values of the 10-, 2-, 1-, 0.2-percent-annual-chance peak discharges were obtained from a log-Pearson Type III distribution of annual peak flow data at these sites in accordance with the guidelines set forth in U.S. Water Resources Council-Bulletin 17 (U.S. Water Resources Council, 1976).

The possibility of using previously developed regional peak discharge frequency relationships was investigated (U.S. Department of the Interior, 1964; 1975). However, these relationships were not used because of additional flood-frequency data available since they were developed, modifications to the accepted methodology of computing flood-frequency data using log-Pearson Type III analysis, and the need for relationships that would more accurately reflect localized conditions. Therefore, new regional relationships of basin characteristics (drainage area and precipitation) to stream flow characteristics (10-, 2-, 1-, 0.2-percent-annual-chance peak discharges) were developed for determining peak discharges at all sites in the study area. Between these values, peak discharges were prorated by distance, which is approximately proportional to drainage area.

Analyses were carried out to establish the peak elevation-frequency relationships for floods of the selected recurrence intervals for each tidal or lacustrine flooding source studied in detail affecting the county.

Elevations for Trosper Lake were developed from a culvert rating on Percival Creek using discharges from the peak discharge-frequency relationships (U.S. Department of the Interior, 1968). Elevations were verified by information supplied by long-time residents of the area.

Elevations for Black Lake are controlled by outlets at the north and south ends of the lake and were derived by hydraulic analyses of Black River and Outlet of Black Lake.

The tidal peak elevation-frequency relationships for Budd Inlet were developed by analyzing 71 years of annual peak tides as recorded at the Seattle Tidal Station by the National Oceanic and Atmospheric Administration or, with the log-Pearson Type III method (U.S. Water Resources Council, 1976), using +0.2 skew. Values of the 10-, 2-, 1- , 0.2-percent-annual-chance tidal peak elevations were then transferred to Budd Inlet and Nisqually Reach by applying adjustments determined from tide prediction tables (U.S. Department of Commerce, 1978) and the high tide of December 15, 1977.

In the City of Yelm, the basin area for the study reach is approximately 9.3 square miles at the upper end and approximately 11.2 miles at the downstream study limit, and varies in elevation from approximately 560 feet in the hills near the City of Rainier to approximately 120 feet at the Nisqually River. Average annual rainfall over the basin is approximately 44 inches. Portions of Yelm Creek run dry in most years, particularly in late summer and early fall. Typical winter flows are low, and appear to result primarily from discharge from the groundwater system. The USGS operated a flow gage on Yelm Creek near the City of Yelm (Gage No. 12089700) from 1968 through 1976. The gage was located in the upper watershed, just downstream of Morris Road, and had a drainage area of 1.7 square miles. Because of the short period of record at the gage and the small portion of the study basin measured, this gage was not applicable to this study.

Peak discharge estimates for the 10-, 2-, and 1-percent-annual-chance floods were computed using USGS regional flood-frequency equations (U.S. Department of the Interior, 1975). The 0.2-percent-annual-chance discharge was determined by estimating the parameters of a log-Pearson Type III fit to the 50-, 10-, 4-, 2-, and 1-percent-annualchance USGS floodflow quantities. This equation was then used to compute the 0.2 percent-annual-chance discharge. This analysis was done using the U.S. Army Corps of Engineers (USACE) PEARSN subroutine (USACE, 1990). Although no significant tributaries enter the study reach, modeled discharges were adjusted at the First Street culvert and 103rd Avenue bridge to reflect the variation in drainage area and contributions by a City storm drain that discharges to Yelm Creek upstream of First Street. No direct measurement of streamflow has ever been made within the study reach of Yelm Creek during a significant flood event. The flood of February 8-9, 1996, ranged between 10- and 0.5-percent-annual-chance events on basins in western Washington. Information from long-time residents of the City of Yelm indicates that flooding along Yelm Creek during the February storm was the worst that had ever been experienced on this reach of Yelm Creek. For purposes of calibration of the hydraulic model, it was assumed that the flow during the February 1996 flood was approximately equal to the 1-percent-annualchance discharge as computed using the USGS regression equations because of the similar hydro-meteorological conditions, regional observations of flooding, and anecdotal information.

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No new hydrologic analyses were conducted as part of this countywide FIS.

Peak discharge-drainage area relationships for the 10-, 4-, 2-, 1-, and 0.2-percentannual- chance floods for each stream studied by detailed methods are presented in Table 6, "Summary of Stillwater Elevations."

Table 6 - Summary of Stillwater Elevations

 $\frac{1}{2}$ water Surface Elevations (Feet NGVD/NAVD₁)

This Physical Map Revision

The peak discharge-frequency relationship for Deschutes River was estimated from analysis of USGS gages and regional regression equations (USGS, 1997). Regional regression equations were used to estimate discharges on Deschutes River upstream of its confluence with Mitchell Creek. From the it's confluence with Michel Creek to Vail Road Crossing, estimated discharges on the Deschutes River are based on an gage weighting analysis of USGS gage 12079000 with regional regression equations. A drainage area-discharge relationship was developed from analysis of USGS gages 12079000 (USGS, 2013a), 12080000 (USGS, 2013b) & 12080010 (USGS, 2013c). This relationship was used to estimate discharges on Deschutes River between Vail Road Crossing and Olympia Brewery Dam. The gage data was analyzed by Bulletin 17B (WRC, 1981) methodology and the log-Pearson Type III distribution, using the USGS PeakFQ computer program (Flynn, et al, 2006). Gage data showed no significant evidence of mixed population.

Peak discharge-drainage area relationships for the 10-, 2-, 1-, and 0.2-percent-annualchance floods for each stream studied by detailed methods are presented in Table 7, "Summary of Discharge

Table 7 – Summary of Discharges

¹Includes effect of overflow from Chehalis River

* Data not available

Table 7 – Summary of Discharges *(continued)*

*Data not available

Table 7 – Summary of Discharges *(continued)* **Peak Discharges (cubic feet per second)**

*Data not available

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the sources studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the FIRM represent rounded whole-foot elevations and may not exactly reflect the elevations shown on the Flood Profiles or in the Floodway Data tables in the FIS report. Flood elevations shown on the FIRM are primarily intended for flood insurance rating purposes. For construction and/or floodplain management purposes, users are cautioned to use the flood elevation data presented in this FIS in conjunction with the data shown on the FIRM.

Flood profiles were drawn showing computed water-surface elevations to an accuracy of 0.5-foot for floods of the selected recurrence intervals. Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1). For stream segments for which a floodway is computed (Section 4.2), selected cross-section locations are also shown on the FIRM (Exhibit 2). Unless specified otherwise, the hydraulic analyses for these studies were based on unobstructed flow. The flood elevations shown on the profiles are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

All elevations shown on the Flood Profiles and FIRM (Exhibits 1 and 2) are referenced to the NAVD88.

Precountywide Analyses

For each incorporated community within Thurston County that had a previously printed FIS report, the hydraulic analyses described in those reports have been compiled and are summarized below.

In the Town of Bucoda, Skookumchuck River was studied by detailed methods.

Water-surface elevations for floods of the selected recurrence intervals were computed through use of the USGS step-backwater computer program (U.S. Department of the Interior, 1976).

Much of the cross section data for the backwater analyses of Skookumchuck River were obtained from aerial photographs taken in April 1977 at a scale of 1:9600 (Walker and Associates, 1977). The underwater portions of the cross sections and the elevations and geometry of the Tono-Bucoda Road Bridge were obtained by field survey.

The approximate analysis in the vicinity of Main and Martina Streets was performed by field survey and engineering judgment.

In the City of Lacey, Woodland Creek was studied by detailed methods.

Water-surface elevations for floods of the selected recurrence intervals were computed for Woodland Creek using a combination of the USGS step-backwater computer program (U.S. Department of the Interior, 1976), and computation of an elevation-discharge recurrence at a culvert ("Measurement of Peak Discharge at Culverts", 1968).

Cross section data used for the backwater analyses for the Cities of Lacey, Olympia, Tumwater; City of Tenino and Thurston County were obtained from aerial photographs taken in April 1977, at a scale of 1:9,600 (Walker and Associates, 1977). These data were supplemented by field measurement of the underwater portions. Elevation data and geometry for bridges, culverts, road overflow, and a few additional channel cross sections were obtained by field survey. The underwater portions of the cross sections, elevations, and geometry of the Draham Street NE culvert were obtained by field survey.

The hydraulic analyses for areas studied by approximate methods were based on flood- depth information, topographic maps (Harl Pugh and Associates, 1978), photographs (Walker and Associates, 1977), and field inspection.

In the City of Olympia, the Outlet of Black Lake was studied by detailed methods.

Water-surface elevations of floods for the selected recurrence intervals were computed through use of a combination of the culvert rating analyses (U.S. Department of the Interior, 1968) and USGS step-backwater computer program (U.S. Department of the Interior, 1976).

Approximate flooding was determined using historical flooding information provided by local residents and field inspection of the area.

In the City of Tenino, Scatter Creek and Scatter Creek Tributary were studied by detailed methods.

Starting water-surface elevations for the first cross section of Scatter Creek and Scatter Creek Tributary (in the City of Tenino) were computed from profile convergence from downstream cross sections and culvert ratings where an approach section was the section farthest downstream.

In Thurston County unincorporated areas, the following streams were studied by detailed methods: Deschutes River, Skookumchuck River, Scatter Creek, Scatter Creek Tributary, Chehalis River, Black River, Outlet of Black Lake, Percival Creek, Woodland Creek, Nisqually River, and Yelm Creek. Nisqually River has been converted to Zone A both in Thurston and Pierce County due to the extreme channel migration that has occurred since the effective models were created.

Water-surface elevations of floods of the selected recurrence intervals for the City of Tumwater; Scatter Creek and Scatter Creek Tributary (in the City of Tenino); and Thurston County were computed through use of a combination of the USGS E-43l step- backwater computer program ("Computer Applications for Step-Backwater", 1976), culvert rating analyses ("Techniques of Water-Resources Investigations", 1968), and computations of road overflows (U.S. Department of the Interior, 1967).

Starting water-surface elevations for the first cross section of Skookumchuck River, Black River, Scatter Creek, Scatter Creek Tributary, and Chehalis River were determined by profile convergence from downstream cross sections. Starting watersurface elevations for Outlet of Black Lake, Percival Creek, and Woodland Creek were determined by flow over dam ratings or culvert ratings, where an approach section was the section farthest downstream. For Deschutes River, starting watersurface elevations were the ending elevations in the City of Tumwater Flood Insurance Study (FEMA, City of Tumwater, 1980).

Due to the meandering nature of the rivers in Thurston County, a profile base line, rather than the actual stream channel, was used to measure the distance between many cross sections on Deschutes River, Skookumchuck River, Scatter Creek, Chehalis River, Black River, and Nisqually River.

The Pacific Northwest River Basins Commission has established standard stationing points in River Miles along Deschutes River, Nisqually River, Skookumchuck River, Chehalis River, and Black River (Pacific Northwest River Basins Commission, 1969). River Mile stationing was not adopted for purposes of this study, however.

The acceptability of all assumed hydraulic factors, cross sections, and hydraulic structure data was verified by computations that duplicated the profiles of the January 1972 flood for Chehalis River, the February 1972 flood for Nisqually River, the January 1974 flood for Deschutes River, and the December 1977 flood for Skookumchuck River.

During a 1-percent-annual-chance flood, Black Lake inundates Black River for approximately 4 miles downstream to Littlerock. In this reach, Black River essentially acts as an extension of Black Lake at the lake elevation of 133 feet until 123rd Avenue SW at Littlerock. Downstream of Littlerock at the Burlington Northern Railroad crossing, Black River flows out of its channel (for approximately 1 mile) southwestward over a small rise, where shallow flooding results. Once crossing this hill, the water collects in a deeper side channel, combining with backwater from a point further downstream along Black River.

Downstream of the Chicago, Milwaukee, St. Paul and Pacific Railroad, 1-percentannual- chance flows from Chehalis River travel northward to Black River. Floodwaters flow through Chehalis Indian Reservation and across $183rd$ Avenue SW, combining with Black River flow. Most inundation is less than 1 foot deep; however, depths exceed 1 foot in the incised channels that connect Chehalis River and Black River. Discharge from this flow does not enter Black River at any one point; therefore, effects from the additional inflow are not substantial on Black River within Thurston County.

The extent of approximate flooding was determined by field observation, stereophotography, and historical flooding observations through interviews with local residents.

In the City of Tumwater, the following streams were studied by detailed methods: Deschutes River, Outlet of Black Lake, and Percival Creek.

Approximate flood boundaries were determined using historical flooding information provided by local residents and field inspection of the area.

In the City of Yelm, Yelm Creek was studied by detailed methods.

Hydrologic and hydraulic analyses were performed to determine flood elevations for the 10-, 2-, 1-, 0.2-percent-annual-chance flows, as well as the 1- and 0.2-percentannual- chance floodplain boundaries and floodway boundary. All detailed hydraulic analyses were computed using the USACE HEC-RAS computer program (USACE, 1997). The flooding is a function of flat topography, a highly vegetated channel, several under-sized culverts and bridges, road fills that encroach on the floodplain and in-stream fences that restrict flows.

Six road-crossing structures, consisting of two culverts and four bridges, influence hydraulic conditions in the study reach. Additional field data were surveyed at each crossing to ensure accurate representation within the HEC-RAS model.

The topography of Yelm Creek and its floodplain is represented in the HEC-RAS model using 28 cross sections surveyed by Northwest Hydraulic Consultants, Inc., in May 1997. The cross sections were extended using topographic mapping at a scale of 1:4,800, with a contour interval of 2 feet (DeGross Aerial Mapping, 1997), taken from aerial photographs flown in January 1997. Several additional cross sections were interpolated to improve the model's stability and accuracy, especially through the bridges and culverts. Vertical control for the surveys and mapping was achieved using four local monuments referenced to Thurston County survey control.

Starting water-surface elevations at the downstream end of the modeled reach were determined using the slope-area method.

The main channel is typically filled with thick grass and brush throughout the study reach, although some small sections are clear of vegetation (U.S. Department of the Interior, 1987; Chow, 1959). In addition to the dense vegetation, many fences cross the channel and floodplain and further restrict flow. The channel banks in many locations are covered with blackberry bushes, while the floodplain varies between cropped pasture and dense brush.

October 16, 2012 Initial Countywide Analyses

For the October 16, 2012, countywide study, all flooding sources studied by detailed methods with were redelineated on new topographic data derived from the 2002 PSLC Bare Earth LiDAR ASCII Points data, developed by TerraPoint, Inc. The LiDAR data has a RMS vertical accuracy of approximately 30 centimeters.

Some approximate study boundaries were adjusted spatially to match current base map information, including the Thurston County 2006 orthophotography, the 2010 GIS road layer, and the 2002 PSLC LiDAR elevation data.

In addition, the Nisqually River special flood hazard area was converted to approximate zone due to the extreme stream channel migration occurring since the original models where developed.

The Deschutes River floodway and floodway data tables were removed, also due to the extreme channel migration within the floodplain.

All qualifying benchmarks within a given jurisdiction that are catalogued by the National Geodetic Survey (NGS) and entered into the National Spatial Reference System (NSRS) as First or Second Order Vertical and have a vertical stability classification of A, B or C are shown and labeled on the FIRM with their 6-character NSRS Permanent Identifier.

Benchmarks catalogued by the NGS and entered into the NSRS vary widely in vertical stability classification. NSRS vertical stability classifications are as follows:

- Stability A: Monuments of the most reliable nature, expected to hold position elevation (e.g. mounted in bedrock)
- Stability B: Monuments which generally hold their position/elevation (e.g. concrete bridge abutment)
- Stability C: Monuments which may be affected by surface ground movements (e.g. concrete monument below frost line)
- Stability D: Mark of questionable or unknown vertical stability (e.g. concrete monument above frost line, or steel witness post)

In addition to NSRS benchmarks, the FIRM may also show vertical control monuments established by a local jurisdiction; these monuments will be shown on the FIRM with the appropriate designations. Local monuments will only be placed on the FIRM if the community has requested that they be included, and if the monuments meet the aforementioned NSRS inclusion criteria.

To obtain current elevation, description, and/or location information for benchmarks shown on the FIRM for this jurisdiction, please contact the Information Services Branch of the NGS at (301) 713-3242, or visit their Web site at www.ngs.noaa.gov.

It is important to note that temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the Technical Support Data Notebook (TSDN) associated with the FIS report and FIRM for this community. Interested individuals may contact FEMA to access these data.

This Physical Map Revision

Water surface elevations of the approximate and 10-, 4-, 2-, 1-, and 0.2-percentannual-chance floods on the Deschutes River were estimated using of the USACE HEC-RAS 4.1.1 computer program (USACE, 2011). Cross sectional geometries for the detailed analysis Deschutes River were comprised of field run survey data and a digital terrain model (DTM) generated from LiDAR data from the Thurston Geodata Center (Thurston Geodata Center, 2011). Surveyed channel sections were transferred upstream and downstream to LiDAR generated cross sections and were blended with the LiDAR data to create a consistent channel profile. Floodway encroachment stations were established, first using Method 4. The Method 4 encroachment stations were imported and the Method 1 encroachment analysis was then executed to create the final floodway.

Starting water surface elevations were calculated bases on normal depth boundary condition.

Channel roughness factors (Manning's "n") used in the hydraulic computations were chosen by respective contractors who performed the original studies.

Channel and overbank roughness factors used in the hydraulic computations were estimated by field observation. Table 8, "Manning's "n" Values", shows the channel and overbank "n" values for the streams studied by detailed methods.

Table 8 – **Manning's "n" Values**

3.3 Vertical Datum

All FIS reports and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum used for newly created or revised FIS reports and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD29). With the completion of the NAVD88, many FIS reports and FIRMs are now prepared using NAVD88 as the referenced vertical datum.

Flood elevations shown in this FIS report and on the FIRM are referenced to the NAVD88. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. Some of the data used in this revision were taken from the prior effective FIS reports and FIRMs and adjusted to NAVD88. The datum conversion factor from NGVD29 to NAVD88 in Thurston County is **3.47 feet**.

The BFEs shown on the FIRM represent whole-foot rounded values. For example, a BFE of 102.4 will appear as 102.0 on the FIRM and 102.6 will appear as 103.0. Therefore, users that wish to convert the elevations in this FIS to NGVD29 should apply the conversion factor to elevations shown on the Flood Profiles and supporting data tables in this FIS report, which are shown at a minimum to the nearest 0.1 foot.

 $NAVD88 = NGVD29 + 3.47$ feet

For additional information regarding conversion between the NGVD29 and NAVD88, visit the NGS website at http://www.ngs.noaa.gov, or contact the NGS at the following address:

Vertical Network Branch, N/CG13 National Geodetic Survey, NOAA Silver Spring Metro Center 3 1315 East-West Highway Silver Spring, Maryland 20910 (301) 713-3191

Temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the TSDN associated with the FIS report and FIRM for this community. Interested individuals may contact FEMA to access these data.

To obtain current elevation, description, and/or location information for benchmarks shown on this map, please contact the Information Services Branch of the NGS at (301) 713-3242, or visit their website at http://www.ngs.noaa.gov.

4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The NFIP encourages State and local governments to adopt sound floodplain management programs. Therefore, each FIS provides 1-percent-annual-chance (100 year) flood elevations and delineations of the 1- and 0.2-percent-annual-chance (500-year) floodplain boundaries and 1- percent-annual-chance floodway to assist communities in developing floodplain management measures. This information is presented on the FIRM and in many components of the FIS report including Flood Profiles and Floodway Data Table. Users should reference the data presented in the FIS report as well as additional information that may be available at the local map repository before making flood elevation and/or floodplain boundary determinations.

4.1 Floodplain Boundaries

To provide a national standard without regional discrimination, the 1-percentannual-chance flood has been adopted by FEMA as the base flood for floodplain management purposes. The 0.2-percent-annual-chance flood is employed to indicate additional areas of flood risk in the community. For each stream studied by detailed methods, the 1- and 0.2-percent-annual-chance floodplain boundaries

have been delineated using the flood elevations determined at each cross section. Between cross sections, the boundaries were interpolated using topographic maps. Approximate flood boundaries were delineated using aerial photographs at a scale of 1:9,600 (Walker and Associates, 1977), topographic maps at a scale of 1:4800, 4 feet contour interval (Harl Pugh and Associates, 1978); Flood Hazard Boundary Maps, and field inspection.

For the October 16, 2012, countywide study, all flooding sources studied by detailed methods with were redelineated on new topographic data derived from the 2002 PSLC Bare Earth LiDAR ASCII Points data, developed by TerraPoint, Inc. The LiDAR data has a RMS vertical accuracy of approximately 30 centimeters. Adjustments were made to approximate flood boundaries as well where necessary to tie into the redelineated detailed flood boundaries.

Before the October 16, 2012, countywide study, the detailed study flood boundaries were delineated on 2 and 4 foot topographic contour maps ranging in scales from 1:1,200 to 1:4,800.

For this physical map revision, the detailed portion of Deschutes River and steams studied by approximate methods were delineated using LiDAR data from the Thurston Geodata Center (Thurston Geodata Center, 2011). Topographic data was converted into a 3 meter digital elevation model (DEM).

The 1- and 0.2-percent-annual-chance floodplain boundaries are shown on the FIRM. On this map, the 1-percent-annual-chance floodplain boundary corresponds to the boundary of the areas of special flood hazards (Zones A and AE), and the 0.2 percent-annual-chance floodplain boundary corresponds to the boundary of areas of moderate flood hazards. In cases where the 1- and 0.2-percent-annual-chance floodplain boundaries are close together, only the 1-percent-annual-chance floodplain boundary has been shown. Small areas within the floodplain boundaries may lie above the flood elevations, but cannot be shown due to limitations of the map scale and/or lack of detailed topographic data.

For the streams studied by approximate methods, only the 1-percent-annual-chance floodplain boundary is shown on the FIRM.

4.2 Floodways

Encroachment on floodplains, such as structures and fill, reduces flood-carrying capacity, increases flood heights and velocities, and increases flood hazards in areas beyond the encroachment itself. One aspect of floodplain management involves balancing the economic gain from floodplain development against the resulting increase in flood hazard. For purposes of the NFIP, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 1-percent-annual-chance floodplain is divided into a floodway and a floodway fringe. The floodway is the channel of a stream, plus any adjacent floodplain areas, that must be kept free of encroachment so that the base flood can be carried without substantial increases in flood heights. Minimum Federal standards limit such increases to 1 foot, provided that hazardous velocities are not produced. The floodways in this study are presented to local agencies as minimum standards that can be adopted directly or that can be used as a basis for additional floodway studies.

The floodways presented in this study were computed for certain stream segments on the basis of equal-conveyance reduction from each side of the floodplain. Floodway widths were computed at cross sections. Between cross sections, the floodway boundaries were interpolated. The results of the floodway computations are tabulated for selected cross sections Table 9, "Floodway Data." In cases where the floodway and 1-percent-annual-chance floodplain boundaries are either close together or collinear, only the floodway boundary is shown.

Encroachment into areas subject to inundation by floodwaters having hazardous velocities aggravates the risk of flood damage, and heightens potential flood hazards by further increasing velocities. A listing of stream velocities at selected cross sections is provided in Table 9, "Floodway Data." In order to reduce the risk of property damage in areas where the stream velocities are high, the community may wish to restrict development in areas outside the floodway.

The floodway for Woodland Creek and a portion of Outlet of Black Lake (from cross sections E to H) coincide with the 1-percent-annual-chance boundary because the channel velocity is high (at or near critical) and the flow is confined to the high-water channel. For these reasons, no information is presented for either Woodland Creek or a portion of Outlet of Black Lake in Table 9.

A floodway is not appropriate along Percival Creek upstream of 54th Avenue SW. This road impounds water from Trosper Lake; thus, there is no conveyance until floodwaters pass through the culvert.

The floodway along Nisqually River was removed for the October 16, 2012, countywide study due to the significant amount of stream channel migration which has occurred since the original flood hazard study was performed.

1 3 Stream Distance in Feet from Chicago, Milwaukee, St. Paul & Pacific Railroad

 3 Elevations Computed Assuming Containment of Right Overbank Losses

 2 Width/Width Within County Limits 4 Width Including Island

TABLE 9

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA**

AND INCORPORATED AREAS

FLOODWAY DATA

CHEHALIS RIVER

¹ Stream Distance in Feet from Chicago, Milwaukee, St. Paul & Pacific Railroad

² Width Including Island

TABLE 9

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA** **FLOODWAY DATA**

AND INCORPORATED AREAS

CHEHALIS RIVER

(Limit of detailed study is approximately 720 feet downstream of Custer Way Southeast)

(Limit of detailed study is approximately 720 feet downstream of Custer Way Southeast)

(Limit of detailed study is approximately 720 feet downstream of Custer Way Southeast)

(Limit of detailed study is approximately 720 feet downstream of Custer Way Southeast)

¹ Stream Distance in Feet Above Mouth

² No Floodway

TABLE **TABLE 9**

 \bullet

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA**

AND INCORPORATED AREAS

FLOODWAY DATA

OUTLET OF BLACK LAKE

¹ Stream Distance in Feet Above Sapp Road

² No Floodway

TABLE **TABLE 9**

 \bullet

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA** AND INCORPORATED AREAS

FLOODWAY DATA

PERCIVAL CREEK

¹ Stream Distance in Feet from Grand Mound Road

TABLE TABLE 9

 \bullet

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA** AND INCORPORATED AREAS

FLOODWAY DATA

SCATTER CREEK

¹ Stream Distance in Feet from Grand Mound Road

TABLE 9 **TABLE 9**

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA**

AND INCORPORATED AREAS

SCATTER CREEK

¹ Stream Distance in Feet from Grand Mound Road

² Stream Distance in Feet Above Mouth

TABLE 9

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA** AND INCORPORATED AREAS

FLOODWAY DATA

SCATTER CREEK and SCATTER CREEK TRIBUTARY

 1 Stream Distance in Feet from State Highway 507 $\,$

² Width/Width Within Thurston County

TABLE

9

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA** AND INCORPORATED AREAS

FLOODWAY DATA

SKOOKUMCHUCK RIVER

¹ Stream Distance in Feet from State Highway 507

TABLE 9 **TABLE 9**

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA**

FLOODWAY DATA

AND INCORPORATED AREAS

SKOOKUMCHUCK RIVER

¹ Stream Distance in Feet from State Highway 507

TABLE 9 **TABLE 9**

FEDERAL EMERGENCY MANAGEMENT AGENCY

FLOODWAY DATA

THURSTON COUNTY, WA AND INCORPORATED AREAS

¹ Stream Distance in Feet from State Highway 507

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA**

AND INCORPORATED AREAS

FLOODWAY DATA

SKOOKUMCHUCK RIVER

TABLE 9

¹ Stream Distance in Feet above Pleasant Glade Road Northeast

² No Floodway

TABLE **TABLE 9**

 \bullet

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA** AND INCORPORATED AREAS

FLOODWAY DATA

WOODLAND CREEK

¹ Stream Distance in Feet From Upstream Face of Centralia Power Canal Flume

 FEDERAL EMERGENCY MANAGEMENT AGENCY **THURSTON COUNTY, WA** AND INCORPORATED AREAS

TABLE **TABLE 9**

 \bullet

FLOODWAY DATA

YELM CREEK

The area between the floodway and 1-percent-annual-chance floodplain boundaries is termed the floodway fringe. The floodway fringe encompasses the portion of the floodplain that could be completely obstructed without increasing the water surface elevation of the 1-percent-annual-chance flood more than 1-foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to floodplain development are shown in Figure 2.

Figure 2 - Floodway Schematic

5.0 INSURANCE APPLICATIONS

For flood insurance rating purposes, flood insurance zone designations are assigned to a community based on the results of the engineering analyses. These zones are as follows:

Zone A

Zone A is the flood insurance risk zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by approximate methods. Because detailed hydraulic analyses are not performed for such areas, no (1-percent-annual-chance) BFEs or base flood depths are shown within this zone.

Zone AE

Zone AE is the flood insurance risk zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by detailed methods. In most instances, whole-foot BFEs derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone X

Zone X is the flood insurance risk zone that corresponds to areas outside the 0.2-percent-annualchance floodplain, areas within the 0.2-percent-annual-chance floodplain, areas of 1-percentannual-chance flooding where average depths are less than 1-foot, areas of 1-percent-annualchance flooding where the contributing drainage area is less than 1 square mile, and areas protected from the 1-percent-annual-chance flood by levees. No BFEs or base flood depths are shown within this zone.

6.0 FLOOD INSURANCE RATE MAP

The FIRM is designed for flood insurance and floodplain management applications.

For flood insurance applications, the map designates flood insurance risk zones as described in Section 5.0 and, in the 1-percent-annual-chance floodplains that were studied by detailed methods, shows selected whole-foot BFEs or average depths. Insurance agents use the zones and BFEs in conjunction with information on structures and their contents to assign premium rates for flood insurance policies.

For floodplain management applications, the map shows by tints, screens, and symbols, the 1 and 0.2-percent-annual-chance floodplains, floodways, and the locations of selected cross sections used in the hydraulic analyses and floodway computations.

The countywide FIRM presents flooding information for the geographic area of Thurston County. Previously, separate Flood Hazard Boundary Maps (FHBMs) and/or FIRMs were prepared for each incorporated community with identified flood hazard areas. Historical map dates relating to pre-countywide maps prepared for each community are presented in Table 10, "Community Map History".

7.0 OTHER STUDIES

This FIS report either supersedes or is compatible with all previous studies published on streams studied in this report and should be considered authoritative for the purposes of the NFIP.

FIS reports were previously published for 8 cities and towns in Thurston County (References FEMA, December 1979; January 1980; March 1981; August 1981; April 1984; and June 1999).

Because it is based on more up-to-date analyses, this FIS supersedes the previously printed FISs for the communities within Thurston County.

8.0 LOCATION OF DATA

Information concerning the pertinent data used in the preparation of this study can be obtained by contacting Federal Insurance and Mitigation Division, FEMA Region X, Federal Regional Center, 130 228th Street, SW, Bothell, Washington 98021-9796.

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